CITY OF BRYAN, TEXAS
CONSTRUCTION PLANS

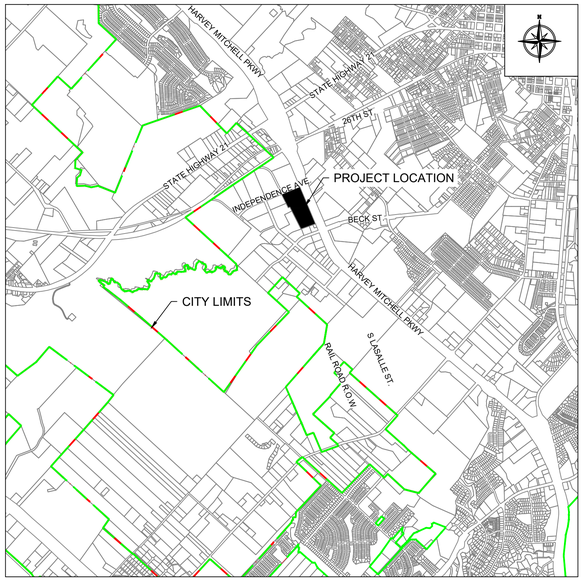
NON-AQUEOUS TOTE STORAGE BUILDING

1331 INDEPENDENCE AVE., BRYAN, TX 77803

FOR

LIQUID POWER FACILITY PRODUCTS, INC.

MBC PROJECT NO. 25-0133



VICINITY MAP

N.T.S.

PREPARED BY:

MBC MANAGEMENT

MBC MANAGEMENT, INC.
NAVASOTA, TEXAS
FIRM NO. 789

SHEET INDEX

SHEET No.	SHEET NAME
C1	PROPOSED SITE GRADING PLAN
C2	PROPOSED SITE GRADING PROFILES
D1	PROPOSED SITE DRAINAGE PLAN
S1	PROPOSED BUILDING PLAN
S2	PROPOSED FOUNDATION PLAN
S3	PROPOSED BUILDING DETAILS

STATE OF TEXAS
PAUL E. MALEK
82860
LICENSED
PROFESSIONAL ENGINEER
DATE: 12/16/2025

THE SEAL APPEARING ON THIS DOCUMENT WAS AUTHORIZED BY PAUL MALEK, P.E. 82860

FIRM NAME:
MBC MANAGEMENT
DESIGN-BUILD | ENGINEERING | GENERAL CONTRACTING

TBPE FIRM NO.: F-789
7984 HWY. 6
NAVASOTA, TX 77868
(P): 936-825-1603
(F): 936-825-1624

PROJECT NAME & LOCATION:
NON-AQUEOUS TOTE STORAGE BUILDING
LIQUID POWER FACILITY PRODUCTS, INC.
1331 INDEPENDENCE AVE., BRYAN, TX 77803

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I. STRUCTURAL SUBGRADE

A. THE TOTAL AREA OF THE FOUNDATION SHALL HAVE THE EXISTING VEGETATION AND TOPSOIL REMOVED. THE SUBGRADE SHALL BE PROOF ROLLED PRIOR TO PLACEMENT OF STRUCTURAL FILL. AREAS FOUND TO BE SUBSTANDARD SHALL BE REMOVED AND REPLACED AS DIRECTED BY THE ENGINEER. THE PREPARATION OF THE SUBGRADE SHALL BE SUBJECT TO REVIEW OF THE ACTUAL FIELD SOIL CONDITIONS BY THE ENGINEER UPON EXPOSURE OF THE SUBGRADE BY SITE EXCAVATION.

B. ALL FILL PLACED UNDER THE FOUNDATION SLAB SHALL HAVE A MAXIMUM PLASTICITY INDEX OF 20 OR LESS. THE MINIMUM PLASTICITY INDEX SHALL BE 5.

C. ALL SUBGRADE SHALL BE PLACED IN A MAXIMUM LIFT THICKNESS OF SIX INCHES. EACH LIFT SHALL BE COMPACTED TO 95% OF STANDARD PROCTOR DENSITY (ASTM D-698) AT A MOISTURE CONTENT OF -1% TO +3% OF OPTIMUM.

D. SUBSURFACE SOIL DESIGN PARAMETERS:

1. DESIGN CLIMATIC RATING - 22

2. DESIGN BEARING STRENGTH - 5,000 psf AT 15 FEET (15ft) BELOW EXISTING SURFACE

3. DESIGN PLASTICITY INDEX - 25

4. DESIGN POTENTIAL VERTICAL RAISE (PVR) - 1"

A / C1 PROPOSED SITE GRADING PLAN

SCALE: 1"=10'-0"

NOTE:
BACKGROUND GOOGLE MAP IS FOR REFERENCE ONLY AND IS NOT TO EXACT SCALE

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DRAFTED BY:	DESIGNED BY:
SGH	PEM

SCALE: AS SHOWN

DRAWING TITLE: PROPOSED SITE GRADING PLAN

DRAWING NO.: C1

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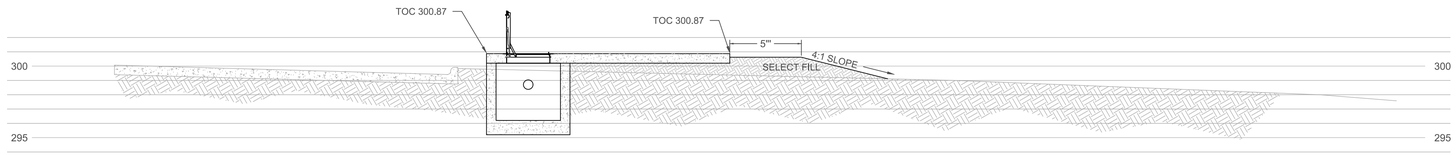
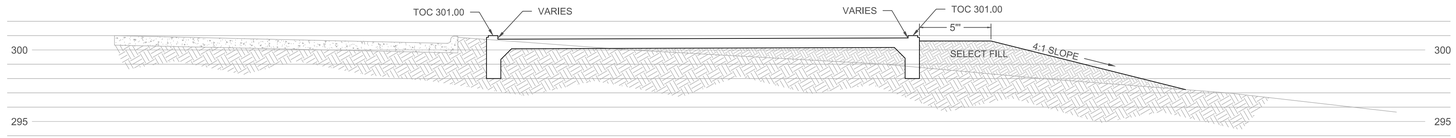


B/C2 PROPOSED SITE PROFILE

SCALE: 1"=5'-0"

C/C2 PROPOSED SITE PROFILE

SCALE: 1"=5'-0"

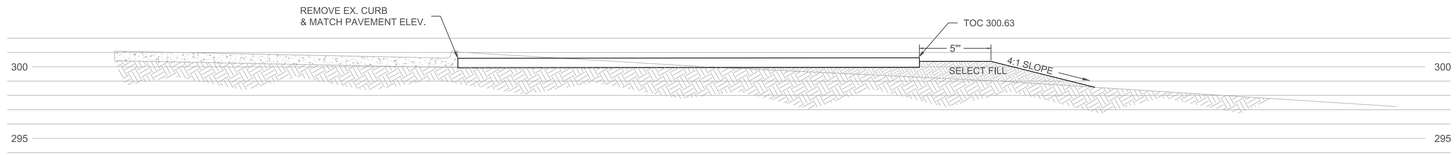


D/C2 PROPOSED SITE PROFILE

SCALE: 1"=5'-0"

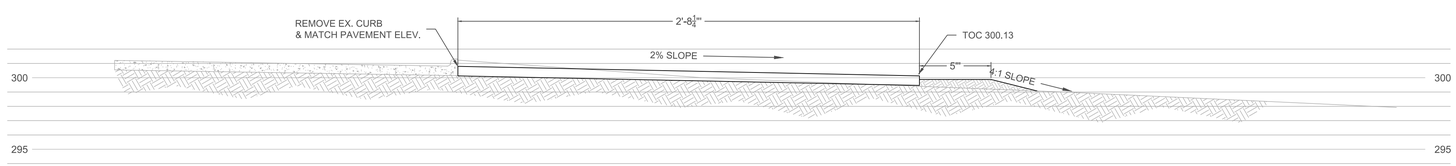
E/C2 PROPOSED SITE PROFILE

SCALE: 1"=5'-0"



F/C2 PROPOSED SITE PROFILE

SCALE: 1"=5'-0"



G/C2 PROPOSED SITE PROFILE

SCALE: 1"=5'-0"

SCALE: 1"=10'-0"

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DRAFTED BY:	DESIGNED BY:
SGH	PEM

SCALE: AS SHOWN

DRAWING TITLE: PROPOSED SITE DRAINAGE PROFILES

DRAWING NO.: C2

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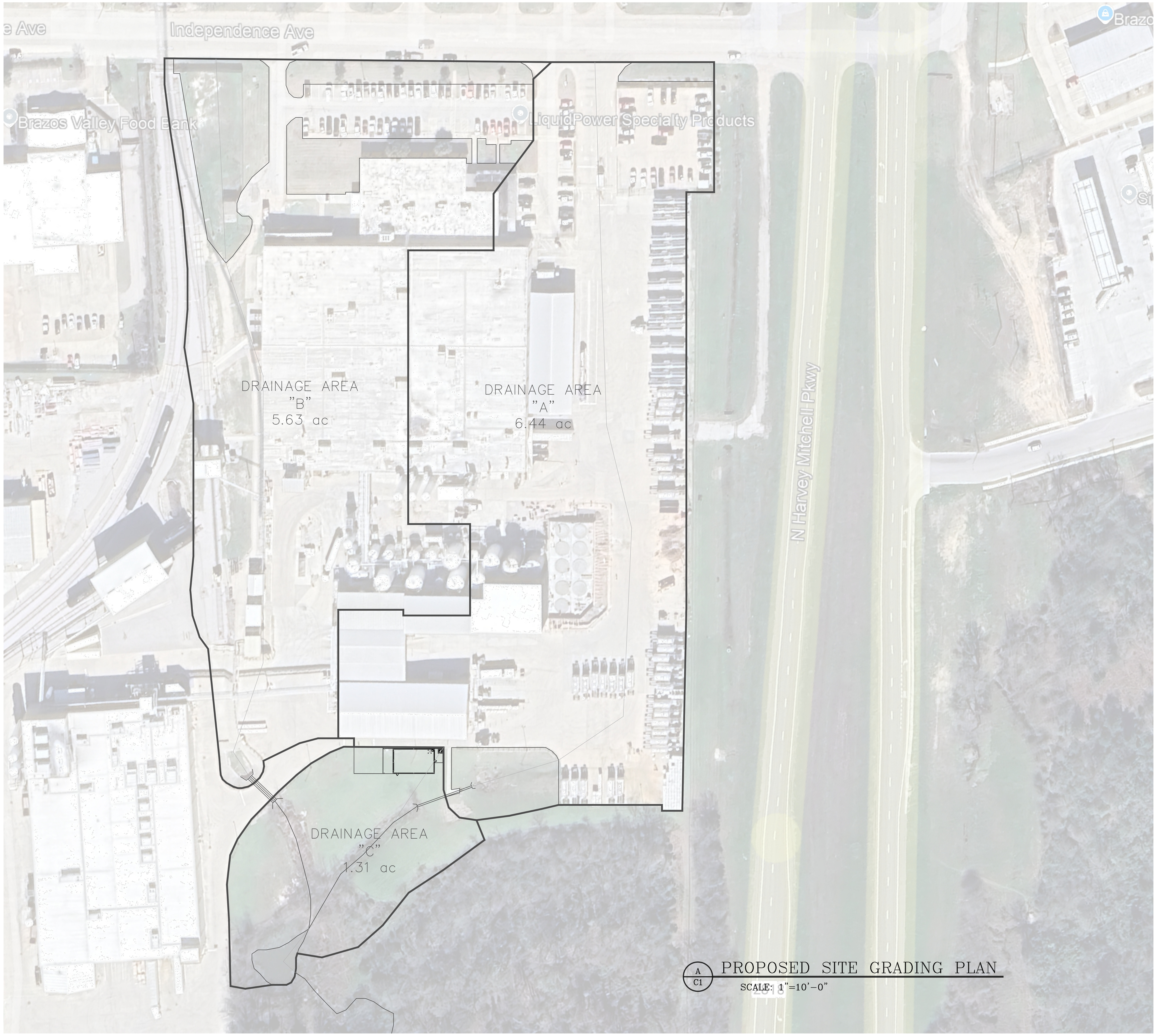


A D1 SITE DRAINAGE PLAN

SCALE: 1"=60'-0"

Atlas-14 Stormwater Intensity

Drainage Area ID	Condition	Time of Concentration	Atlas-14 Stormwater Flow (in/hr)			
		min	2-yr	10-yr	25-yr	100-yr
A	Pre	12.34	4.39	6.45	7.57	11.23
B	Pre	7.59	5.23	7.64	8.94	13.08
C	Pre	12.72	4.34	6.38	7.48	11.11
A	Post	12.34	4.39	6.45	7.57	11.23
B	Post	7.59	5.23	7.64	8.94	13.08
C	Post	11.31	4.55	6.67	7.82	11.58

Existing CN Calculation

Drainage Area ID	Pasture, Type "D" Soil	Impervious Areas	Total Area	Average
	AC 69	AC 98	AC	CN
A	0.3	6.14	6.44	96.65
B	0.87	4.76	5.63	93.52
C	1.31	0	1.31	69.00

Existing Time of Concentration

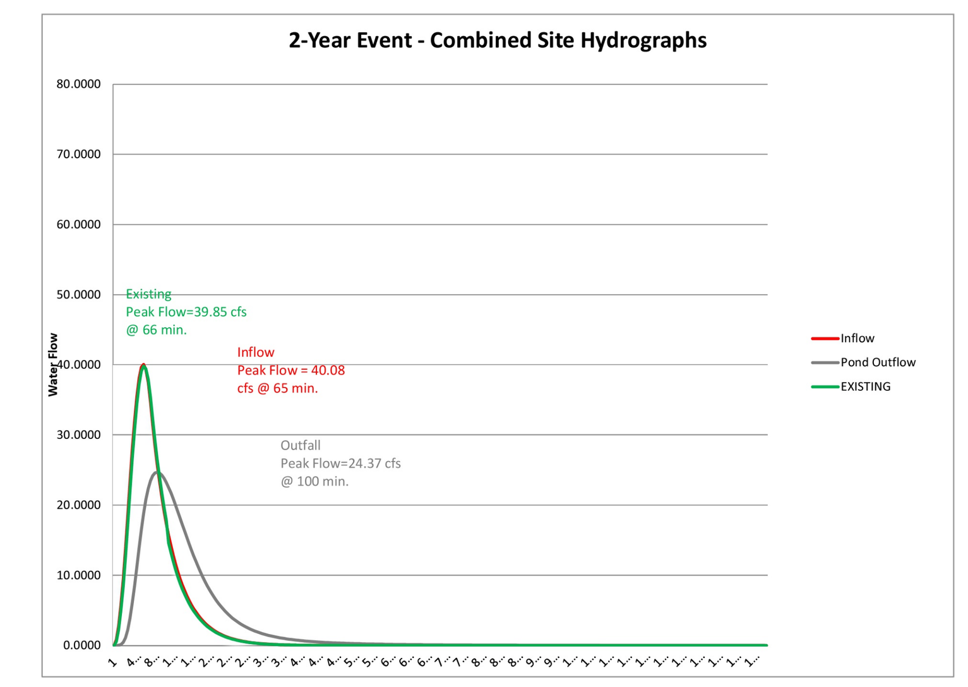
	Overland Sheet Flow						Shallow Concentrated Flow					Channel Flow				Total	Total	Total
Drainage Area ID	n	Length	P2*	Slope	Velocity	Overland Travel Time	Slope	Distance	Surface ("Paved" of "Unpaved")	Velocity	Shallow Concentrated Flow Travel Time	Slope	Veloocity	Distance	Channel Travel Time	Travel Distance	Time of Concentration	Lag Time
		ft	inches	%	ft/s	min	%	ft	type	ft/s	min	%	ft/s	ft	min	ft	min	min
A	0.05	889	4.5	0.03	0.85	17.43	0.68	110	Unpaved	0.60	3.06	1.50	9.70	70.00	0.12	1069.00	20.61	12.34
B	0.05	640	4.5	0.63	1.50	7.11	1.21	247	Unpaved	0.75	5.49	1.50	9.70	42.00	0.07	929.00	12.67	7.59
C							0.29	344	Unpaved	0.27	21.23					344.00	21.23	12.72

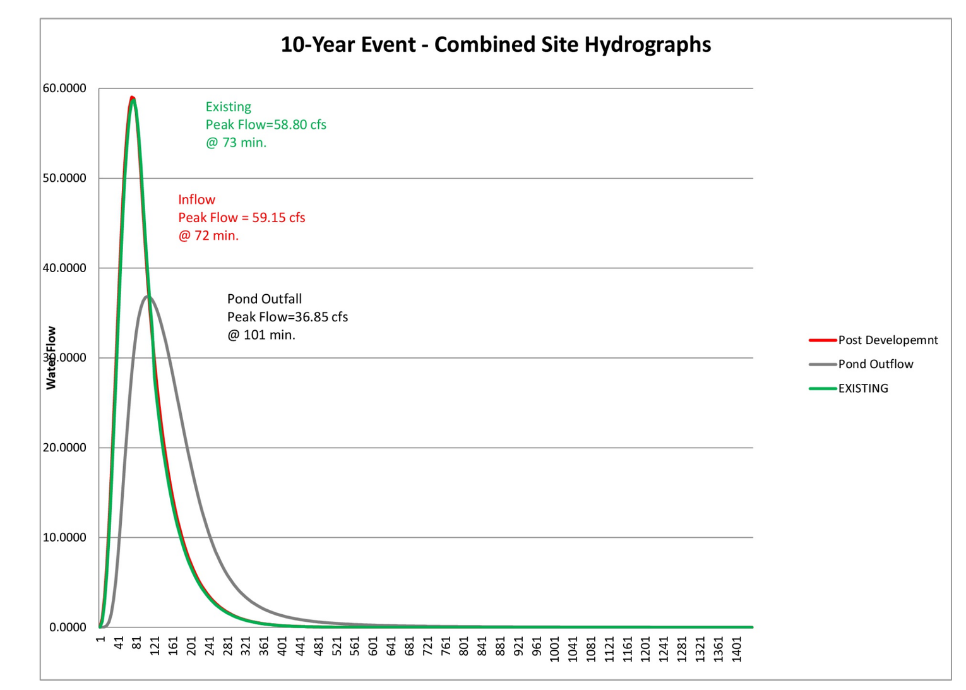
Proposed CN Calculation

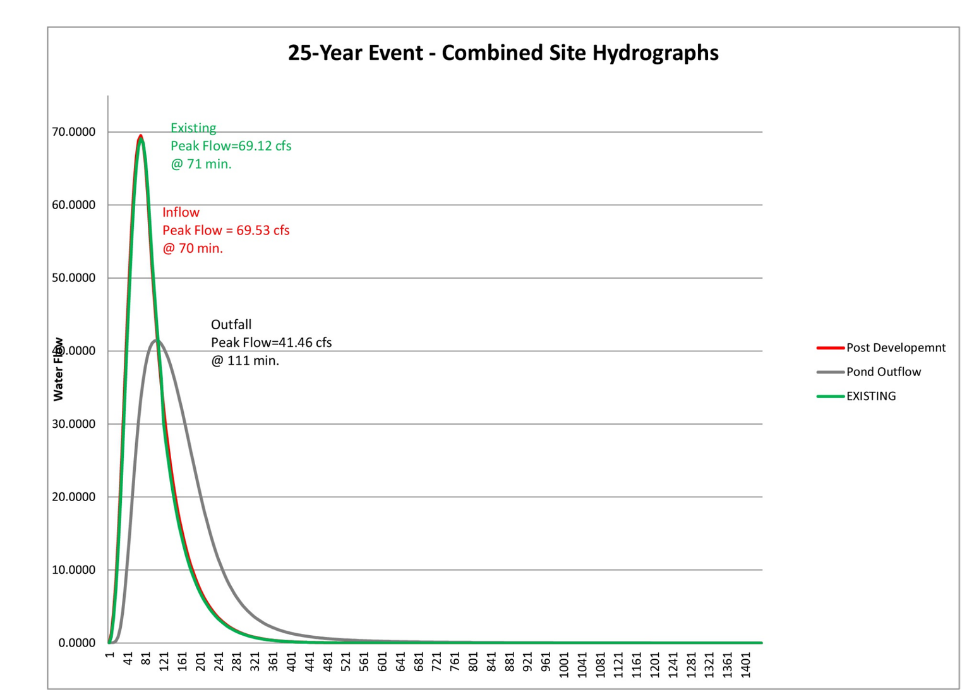
Drainage Area ID	Pasture, Type "D" Soil	Impervious Areas	Total Area	Average
	AC 69	AC 98	AC	CN
A	0.3	6.14	6.44	96.65
B	0.87	4.76	5.63	93.52
C	1.23	0.08	1.31	70.77

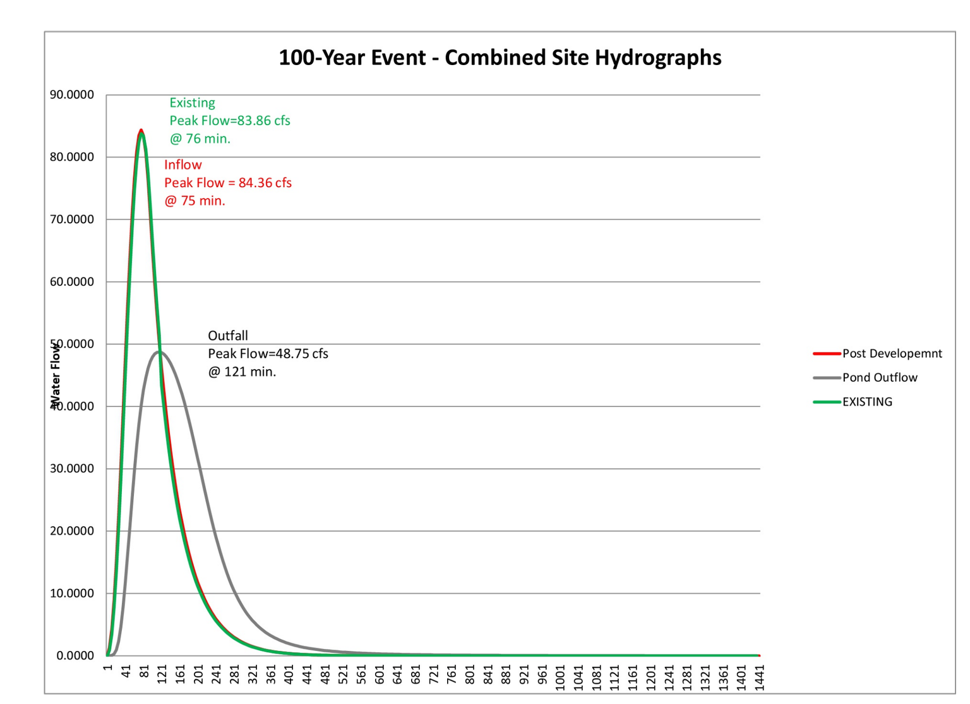
Proposed Time of Concentration

	Overland Sheet Flow						Shallow Concentrated Flow					Channel Flow				Total	Total	Total
Drainage Area ID	n	Length	P2*	Slope	Velocity	Overland Travel Time	Slope	Distance	Surface ("Paved" of "Unpaved")	Velocity	Shallow Concentrated Flow Travel Time	Slope	Veloocity	Distance	Channel Travel Time	Travel Distance	Time of Concentration	Lag Time
		ft	inches	%	ft/s	min	%	ft	type	ft/s	min	%	ft/s	ft	min	ft	min	min
A	0.05	889	4.5	0.03	0.85	17.43	0.68	110	Unpaved	0.60	3.06	1.50	9.70	70.00	0.12	1069.00	20.61	12.34
B	0.05	640	4.5	0.63	1.50	7.11	1.21	247	Unpaved	0.75	5.49	1.50	9.70	42.00	0.07	929.00	12.67	7.59
C	0.05	38	4.5	2.00	1.00	0.63	0.29	306	Unpaved	0.27	18.89					344.00	19.52	11.69









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NON-AQUEOUS TOTE STORAGE BUILDING

DRAFTED BY: SGH | DESIGNED BY: PEM

SCALE: AS SHOWN

DRAWING TITLE: PROPOSED SITE DRAINAGE PLAN

DRAWING NO.: D1

PRE- ENGINEERED METAL BUILDING

A. RIGID FRAMES: ASTM A 529. PRE-ENGINEERED METAL BUILDING FRAMES TO SPAN AND ROOF SLOPES SHOWN AND DESIGNED TO SUPPORT DEAD LOADS INDICATED, LIVE LOADS OF 20 psf AND WIND LOADS AS SET FORTH BY THE 2015 EDITION OF THE I.B.C.

B. DESIGN:
- 1. GENERAL:
  - A. ALL STRUCTURAL STEEL MEMBERS AND WELDED PLATE MEMBERS SHALL BE DESIGNED IN ACCORDANCE WITH THE LATEST EDITION OF AISC SPECIFICATIONS.
  - B. ALL LIGHT-GAGE, COLD FORMED STRUCTURAL MEMBERS AND COVERINGS SHALL BE DESIGNED IN ACCORDANCE WITH THE LATEST EDITION OF THE AISI SPECIFICATIONS.
  - C. DESIGN SHALL BE COMPLETED AND SEALED BY A PROFESSIONAL ENGINEER.
- 2. DESIGN LOADS:
  - A. LIVE LOAD:
    - 1. ROOF LIVE LOAD: 20 psf.
  - B. DEAD LOAD:
    - 1. 5.0 psf IN ADDITION TO STEEL BUILDING D.L. (COLLATERAL LOAD)
  - C. WIND LOAD:
    - 1. MEET OR EXCEED INTERNATIONAL BUILDING CODE.
  - D. UPLIFT (APPLIED UPWARDS TO THE ROOF SYSTEMS IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE.)
  - E. EQUIPMENT LOAD AS REQUIRED TO SUPPORT THE EQUIPMENT SHOWN.
  - F. SEISMIC LOAD:
    - 1. MEET OR EXCEED INTERNATIONAL BUILDING CODE.
  - G. SNOW LOAD:
    - 1. MEET OR EXCEED INTERNATIONAL BUILDING CODE.
  - H. DESIGN LOAD COMBINATIONS:
    - 1. DEAD + FLOOR LIVE + ROOF LIVE (OR SNOW).
    - 2. DEAD + FLOOR LIVE + WIND LOAD (OR SEISMIC).
    - 3. DEAD + FLOOR LIVE + WIND + 1/2 SNOW.
    - 4. DEAD + FLOOR LIVE + 1/2 WIND + SNOW.
    - 5. DEAD + FLOOR LIVE + SNOW + SEISMIC.

C. MEMBERS AND CONNECTIONS:
1. HOT-ROLLED STRUCTURAL SHAPES: 36 ksi, ASTM A 36.
2. PLATE AND BAR STOCK MEMBERS: 42 ksi, ASTM A 529.
3. COLD FORMED MEMBERS: 50 ksi, ASTM A 607 (GRADE 50).
4. ROD BRACING: 36 ksi, ASTM A 36.
5. MISCELLANEOUS MEMBERS: 42 ksi.
6. PRIMARY BOLTED CONNECTIONS: ASTM A 325.
7. SECONDARY BOLTED CONNECTIONS: ASTM A 307.
8. SHOP CONNECTIONS: AWS STRUCTURAL WELDING CODE.

D. MEMBER COATINGS:
1. OXIDE PRIMER.

E. ROOFING MATERIAL:
1. "PBR" ROOF PANEL.
2. 26 ga. GALV. METAL
3. 3" ROOF INSULATION w/ REINFORCED VINYL FACING.

F. WALL MATERIAL:
1. "R" WALL PANEL.
2. 26 ga. COLOR METAL
3. COLOR AS PER OWNER.
4. 3" WALL INSULATION w/ REINFORCED VINYL FACING.

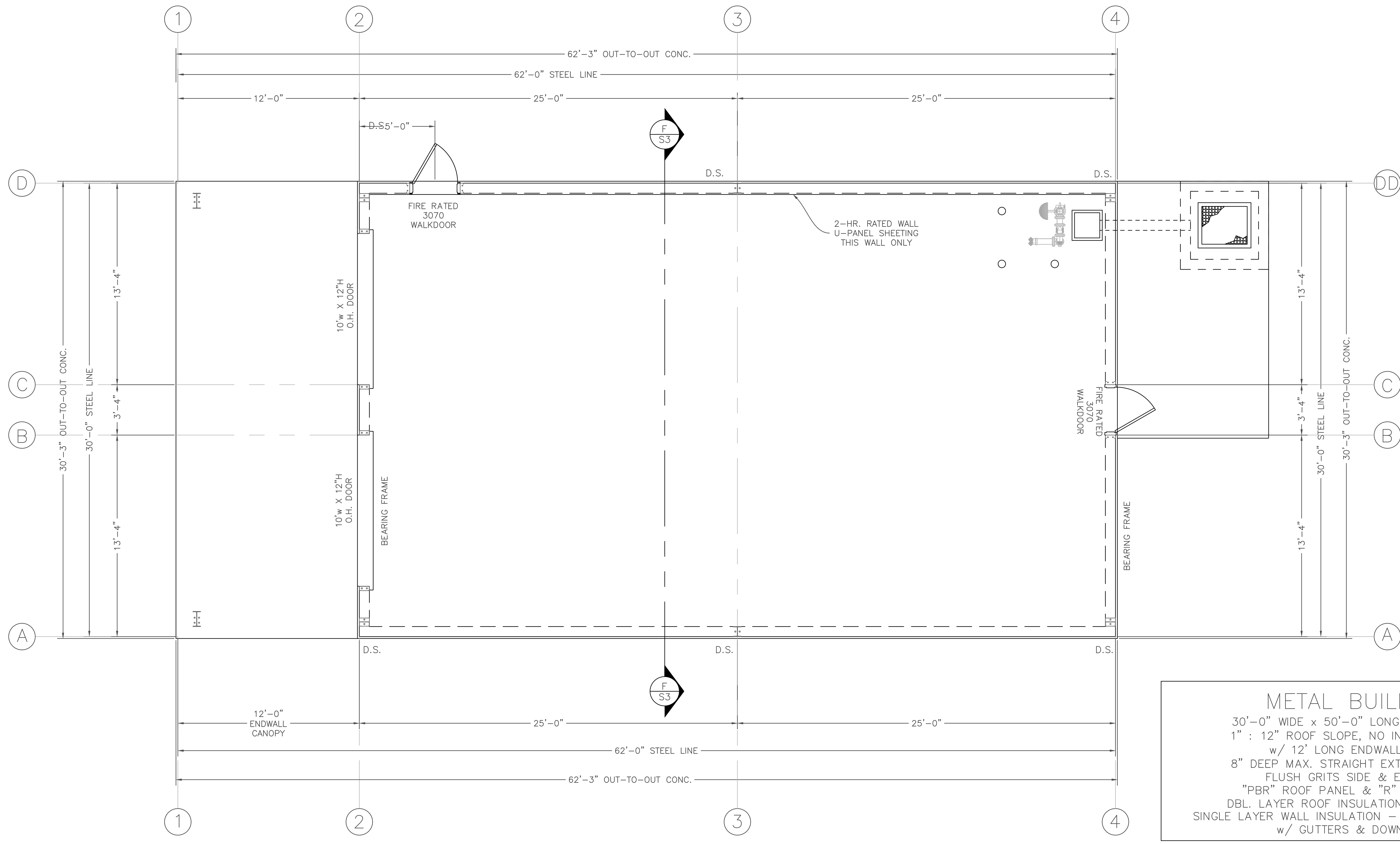
G. TRIM MATERIAL:
1. GUTTERS AND DOWNSPOUTS.
2. CORNERS & OPENINGS TRIM.
3. COLOR AS PER OWNER.

BUILDING CODE:
1. 2021 EDITION OF THE INTERNATIONAL BUILDING CODE.
2. ASCE-7, MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES
3. PROPOSED BUILDING CATEGORY: S-1, FIRE SPRINKLED
   - a. TYPE OF CONSTRUCTION: III-B
   - b. ALLOWABLE No. OF STORIES: 4 — ACTUAL: 1 -25%
   - c. ALLOWABLE BUILDING AREA: 70,000 sf — ACTUAL: 1,860 SF -91%
   - d. ALLOWABLE BUILDING HEIGHT: 65 ft — ACTUAL: 15.25 ft -23.6%
4. IMPORTANCE FACTOR: 1.0
5. FIRE RESISTANCE RATING REQUIREMENTS FOR EXTERIOR WALLS:
   - a. FIRE SEPARATION DISTANCE – GREATER THAN 30ft – 0 hr (TABLE 602)
   - b. RATING OF EXISTING EXTERIOR BUILDING WALL – 1 hr
6. MAXIMUM AREA OF EXTERIOR WALL OPENINGS:
   - a. UNPROTECTED, SPRINKLED — NOT REQUIRED (TABLE 705.8)

LOADS:
1. DEAD LOAD = 2 PSF.
2. ROOF LIVE LOAD = 20 PSF. (REDUCIBLE)
3. GROUND SNOW LOAD = 5 PSF
4. ROOF SNOW LOAD = 5 PSF
5. COLLATERAL LOAD = 5 PSF
6. RAIN LOAD = 4.5 INCHES PER HOUR
7. WIND LOAD:
   - BASIC WIND SPEED = 115 MPH.
   - WIND IMPORTANCE FACTOR = 1.0
   - WIND EXPOSURE "C"
   - VELOCITY PRESSURE = 28.14 PSF
   - WINDWARD HORIZONTAL LOAD = -19.60 PSF
   - WINDWARD UPLIFT LOAD = 14.35 PSF
   - LEEWARD UPLIFT LOAD = 8.12 PSF
   - LEEWARD HORIZONTAL LOAD = 6.62 PSF
   - COMPONENTS AND CLADDING LOAD = 34.34 PSF
5. SEISMIC LOAD:
   - SEISMIC USE GROUP: II
   - SITE CLASS: D, STIFF SOIL
   - SPECTRAL RESPONSE COEFFICIENTS
     - SDS = 0.071
     - SD1 = 0.065
   - ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE

SPECIAL INSPECTION:

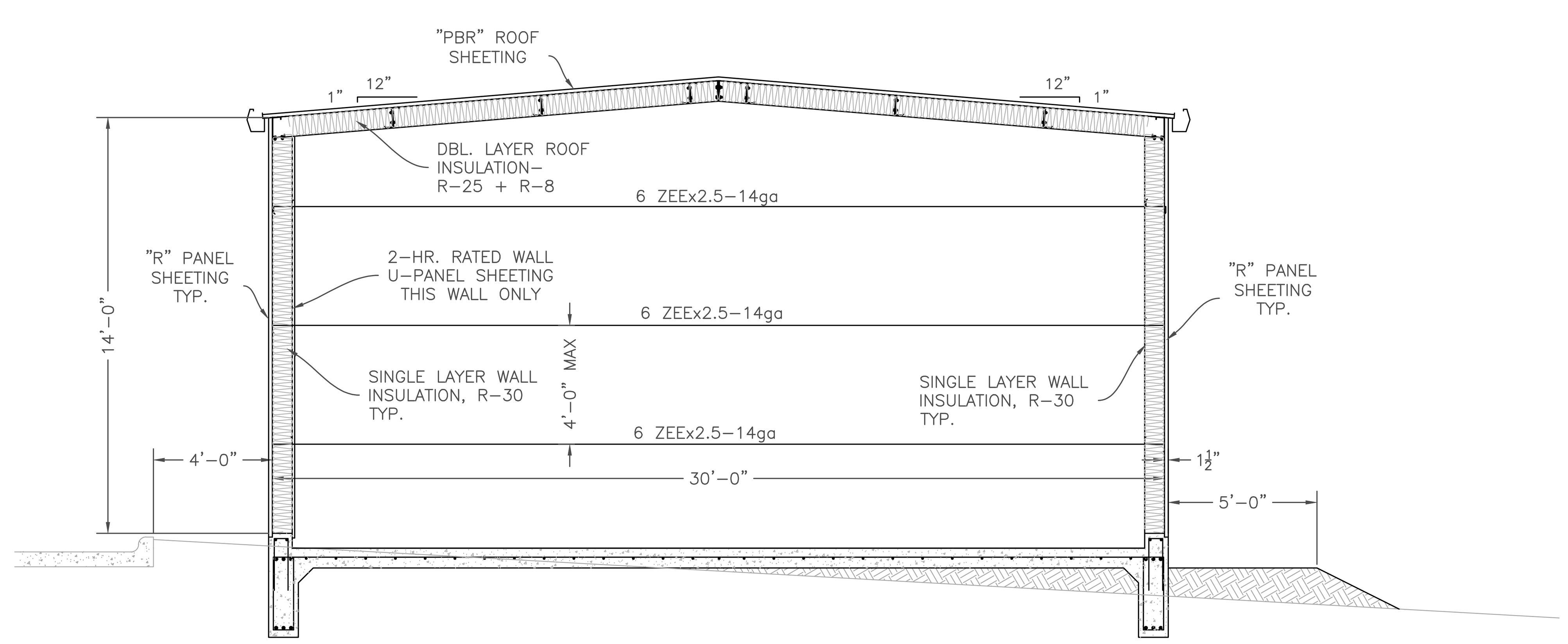
1. PER IBC 2015 SECTION 1704 & 1705, SPECIAL INSPECTION IS REQUIRED FOR THE FOLLOWING ITEMS:
   - A. CONCRETE:
     - A1) DURING THE TAKING OF TEST SPECIMENS.
     - A2) DURING THE PLACEMENT OF ALL REINFORCED CONCRETE, UNLESS NOTED OTHERWISE.
     - A3) NO INSPECTION IS REQUIRED FOR SLABS ON GRADE OR FOR CONCRETE WITH DESIGN COMPRESSIVE STRENGTH OF 2,500 PSI OR LESS.
   - B. BOLTS IN CONCRETE:
     - B1) DURING THE PLACEMENT OF CONCRETE AROUND BOLTS.
   - C. REINFORCING STEEL(PERIODIC):
     - C1) DURING THE PLACING OF REINFORCING STEEL FOR ALL CONCRETE REQUIRED TO HAVE SPECIAL INSPECTION NOTED ABOVE.
   - D. WELDING:
     - D1) VISUAL INSPECTION OF ALL FIELD WELDS.
   - E. HIGH STRENGTH BOLTING:
     - E1) VERIFICATION OF SNUG TIGHT BOLT INSTALLATION FOR A325N BOLTS.
   - F. MASONRY (PERIODIC)
     - F1) DURING PREPARATION OF PRISMS.
     - F2) DURING PLACEMENT OF REINFORCING AND GROUT.
     - F3) CLEAN OUTS PRIOR TO CLOSING.
2. DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR:
   - A. THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED TO BE CERTAIN IT CONFORMS WITH THE APPROVED DESIGN DRAWINGS AND SPECIFICATION.
   - B. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, AND TO THE ENGINEER OR ARCHITECT OF RECORD. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN, IF UNCORRECTED, TO THE DESIGN AUTHORITY AND THE BUILDING OFFICIAL.
   - C. UPON COMPLETION OF THE ASSIGNED WORK THE ENGINEER OR ARCHITECT SHALL COMPLETE AND SIGN THE APPROPRIATE FORMS CERTIFYING THAT TO THE BEST OF HIS KNOWLEDGE THE WORK IS IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS, AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE CODE.



METAL BUILDING
30'-0" WIDE x 50'-0" LONG x 14'-0" EAVE
1" : 12" ROOF SLOPE, NO INTERIOR COLUMN
w/ 12' LONG ENDWALL CANOPY
8" DEEP MAX. STRAIGHT EXTERIOR COLUMNS
FLUSH GRITS SIDE & END WALLS
"PBR" ROOF PANEL & "R" PANEL WALLS
DBL. LAYER ROOF INSULATION- R-25 + R-8
SINGLE LAYER WALL INSULATION – R-30 BETWEEN GIRTS
w/ GUTTERS & DOWNSPOUTS

A / S1 METAL BUILDING PLAN

SCALE: 1/4""=1'-0"



B / S1 BUILDING SECTION

SCALE: 1/4""=1'-0"

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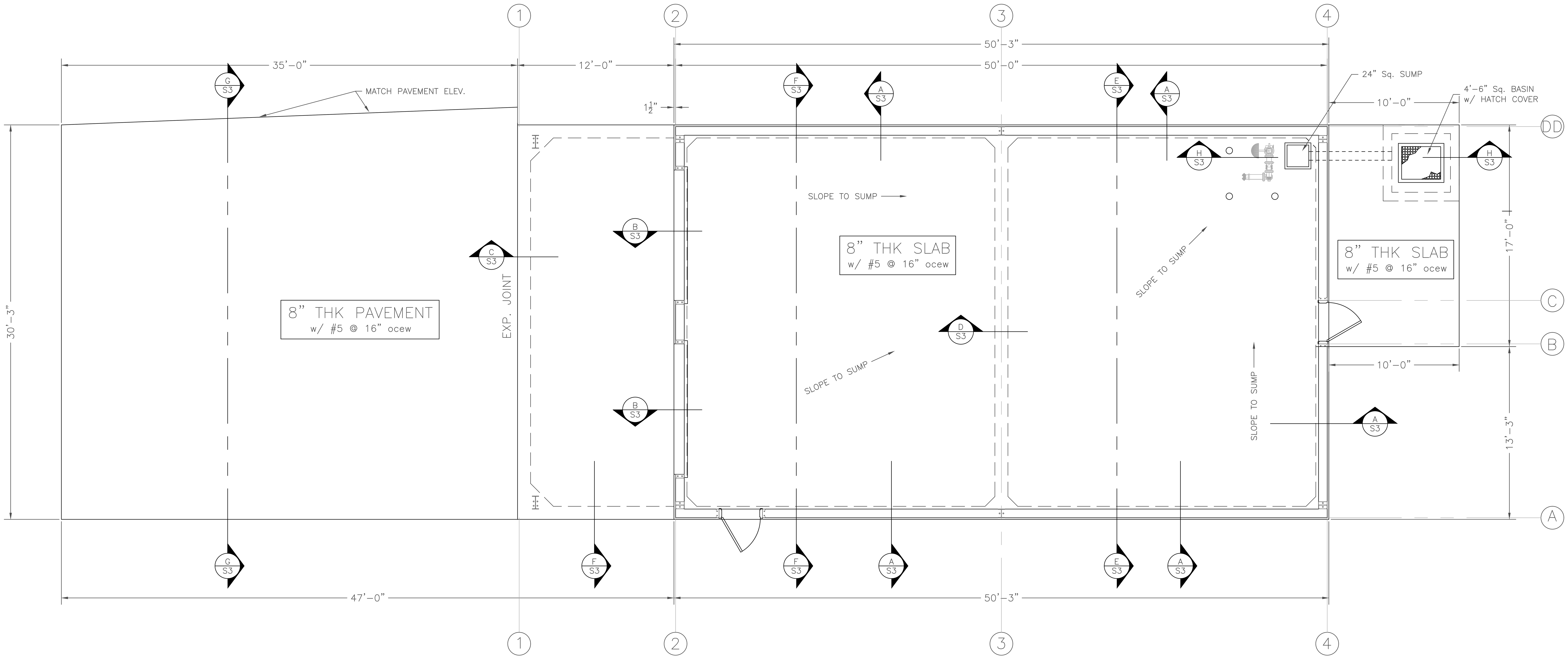


A
S2
FOUNDATION PLAN
SCALE: 1/4""=1'-0"

I. GENERAL

A. ALL FOUNDATION PLAN DIMENSIONS ARE INTERPRETED FROM AND SHALL BE VERIFIED WITH THE ARCHITECTURAL PLANS AND NOTIFY THE OWNER IF DISCREPANCIES EXIST.

B. A 10 mil. POLY VAPOR BARRIER SHALL BE PLACED UNDER ALL CONCRETE. TEARS IN THE VAPOR BARRIER SHALL BE REPAIRED.

C. EXTERIOR BEAMS SHALL BE AS SHOWN EXCEPT DEPTHS SHALL BE INCREASED AS NECESSARY TO PENETRATE A MINIMUM OF 12 INCHES INTO UNDISTURBED SOIL.

D. INTERIOR BEAM LOCATIONS MAY BE SHIFTED NOT MORE THAN SIX INCHES WHERE CONFLICTS WITH PLUMBING LAYOUTS OCCUR.

E. BEAM TRENCHES SHALL BE CLEANED OF DEBRIS AND STANDING WATER PRIOR TO CONCRETE PLACEMENT.

F. ANY UNUSUAL CONDITIONS ENCOUNTERED SHALL BE BROUGHT TO THE ATTENTION OF THE OWNER PRIOR TO CONCRETE PLACEMENT.

G. LOCATIONS OF CONSTRUCTION JOINTS NOT SHOWN SHALL BE APPROVED BY THE ENGINEER PRIOR TO CONCRETE PLACEMENT.

II. CONCRETE

A. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI AT 28 DAYS IN ACCORDANCE WITH ASTM C-39

B. ALL CONCRETE SHALL HAVE A MINIMUM MODULUS OF RUPTURE OF 450 PSI AT 28 DAYS IN ACCORDANCE WITH ASTM C-78.

C. THE MAXIMUM WATER / CEMENT RATIO SHALL BE 0.45.

D. AN AIR ENTRAINMENT AGENT SHALL BE USED.

E. AN MID-RANGE WATER REDUCER MAY BE USED.

F. THE MAXIMUM ALLOWABLE CONCRETE SLUMP SHALL BE 5 INCHES PER ASTM C-143.

G. APPLY A FLOAT FINISH THEN APPLY A TROWEL FINISH THEN A LIGHT BROOM FINISH TO SURFACES TO BE LEFT EXPOSED.

H. TOOL ALL EXPOSED EDGES UNLESS SHOWN OTHERWISE.

I. DEPRESSIONS BETWEEN HIGH SPOTS SHALL NOT BE GREATER THAN 1/8 in. BELOW A 10 ft. LONG STRAIGHTEDGE IN ACCORDANCE WITH ACI 302.

J. CONCRETE FACES SHALL NOT DEVIATE MORE THAN 3/16" FROM THE PLAN DIMENSIONS.L

K. ANCHOR BOLTS SIZE AND LOCATION PER THE BUILDING MANUFACTURE.

III. REINFORCING STEEL

A. ALL REINFORCING STEEL SHALL BE ASTM A-615, GRADE 60.

B. WHERE CONCRETE IS PLACED AGAINST FORMS REINFORCING BARS SHALL HAVE A MINIMUM OF 2 INCHES CLEAR COVER UNLESS SHOWN OTHERWISE. WHERE CONCRETE IS PLACED AGAINST EARTH, REINFORCING BARS SHALL HAVE A MINIMUM OF 3 INCHES CLEAR COVER.

C. DETAILING OF REINFORCING SHALL BE IN ACCORDANCE WITH ACI 315, LATEST EDITION, UNLESS SHOWN OTHERWISE. PLACING OF REINFORCING SHALL BE IN ACCORDANCE WITH C.R.S.I., " RECOMMENDED PRACTICE FOR PLACING OF REINFORCING BARS", LATEST EDITION.

D. WHERE IT IS NECESSARY TO SPLICE REINFORCEMENT AT LOCATIONS OTHER THAN THOSE SHOWN ON THE DRAWINGS, THE SPLICE LOCATIONS SHALL BE APPROVED BY THE ENGINEER. LAP SPLICED AND EMBEDMENT LENGTHS, NOT SHOWN ON THE DRAWINGS, SHALL BE IN ACCORDANCE WITH C.R.S.I., LATEST EDITION.

E. REINFORCING BARS SHALL BE SECURED AT EVERY OTHER INTERSECTION MINIMUM.

F. FOUR CORNER BARS SHALL BE SECURELY TIED TO THE INTERSECTION BEAMS BARS AT ALL OUTSIDE CORNERS AND EXTERIOR BEAM INTERSECTIONS. RHE BARS SHALL BE #5 BARS WITH 2'-0" LEGS MIN.

IV. STRUCTURAL SUBGRADE

A. THE TOTAL AREA OF THE FOUNDATION SHALL HAVE THE EXISTING VEGETATION AND TOPSOIL REMOVED AND AN ADDITIONAL TWENTY-FOUR INCHES (24") EXISTING SOIL TO BE REPLACED WITH SELECT FILL. THE SUBGRADE SHALL BE PROOF ROLLED PRIOR TO PLACEMENT OF STRUCTURAL FILL. AREAS FOUND TO BE SUBSTANDARD SHALL BE REMOVED AND REPLACED AS DIRECTED BY THE ENGINEER. THE PREPARATION OF THE SUBGRADE SHALL BE SUBJECT TO REVIEW OF THE ACTUAL FIELD SOIL CONDITIONS BY THE ENGINEER UPON EXPOSURE OF THE SUBGRADE BY SITE EXCAVATION.

B. ALL FILL PLACED UNDER THE FOUNDATION SLAB SHALL HAVE A MAXIMUM PLASTICITY INDEX OF 20 OR LESS. THE MINIMUM PLASTICITY INDEX SHALL BE 5.

C. ALL SUBGRADE SHALL BE PLACED IN A MAXIMUM LIFT THICKNESS OF SIX INCHES. EACH LIFT SHALL BE COMPACTED TO 95% OF STANDARD PROCTOR DENSITY (ASTM D-698) AT A MOISTURE CONTENT OF -1% TO +3% OF OPTIMUM.

D. SUBSURFACE SOIL DESIGN PARAMETERS:

1. DESIGN CLIMATIC RATING - 22
2. DESIGN BEARING STRENGTH - 5,000 psf AT 15 FEET (15ft) BELOW EXISTING SURFACE
3. DESIGN PLASTICITY INDEX - 25
4. DESIGN POTENTIAL VERTICAL RAISE (PVR) - 1"

SPECIAL INSPECTION:

1. PER IBC 2015 SECTION 1704 & 1705, SPECIAL INSPECTION IS REQUIRED FOR THE FOLLOWING ITEMS:
   A. CONCRETE:
      A1) DURING THE TAKING OF TEST SPECIMENS.
      A2) DURING THE PLACEMENT OF ALL REINFORCED CONCRETE, UNLESS NOTED OTHERWISE.
      A3) NO INSPECTION IS REQUIRED FOR SLABS ON GRADE OR FOR CONCRETE WITH DESIGN COMPRESSIVE STRENGTH OF 2,500 PSI OR LESS.
   B. BOLTS IN CONCRETE:
      B1) DURING THE PLACEMENT OF CONCRETE AROUND BOLTS.
   C. REINFORCING STEEL(PERIODIC):
      C1) DURING THE PLACING OF REINFORCING STEEL FOR ALL CONCRETE REQUIRED TO HAVE SPECIAL INSPECTION NOTED ABOVE.
   D. WELDING:
      D1) VISUAL INSPECTION OF ALL FIELD WELDS.
   E. HIGH STRENGTH BOLTING:
      E1) VERIFICATION OF SNUG TIGHT BOLT INSTALLATION FOR A325N BOLTS.
   F. MASONRY (PERIODIC)
      F1) DURING PREPARATION OF PRISMS.
      F2) DURING PLACEMENT OF REINFORCING AND GROUT.
      F3) CLEAN OUTS PRIOR TO CLOSING.
2. DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR:
   A. THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED TO BE CERTAIN IT CONFORMS WITH THE APPROVED DESIGN DRAWINGS AND SPECIFICATION.
   B. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, AND TO THE ENGINEER OR ARCHITECT OF RECORD. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN, IF UNCORRECTED, TO THE DESIGN AUTHORITY AND THE BUILDING OFFICIAL.
   C. UPON COMPLETION OF THE ASSIGNED WORK THE ENGINEER OR ARCHITECT SHALL COMPLETE AND SIGN THE APPROPRIATE FORMS CERTIFYING THAT TO THE BEST OF HIS KNOWLEDGE THE WORK IS IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS, AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE CODE.

NO.	REVISION DESCRIPTION	DATE	BY

STATE OF TEXAS
PAUL E. MALEK
82860
LICENSED PROFESSIONAL ENGINEER
DATE: 12/16/2025
THE SEAL APPEARING ON THIS DOCUMENT WAS AUTHORIZED BY PAUL MALEK, P.E. 82860

FIRM NAME:
MBC MANAGEMENT
DESIGN-BUILD | ENGINEERING | GENERAL CONTRACTING
TBPE FIRM NO.: F-789
7984 HWY. 6
NAVASOTA, TX 77868
(P): 936-825-1603
(F): 936-825-1624

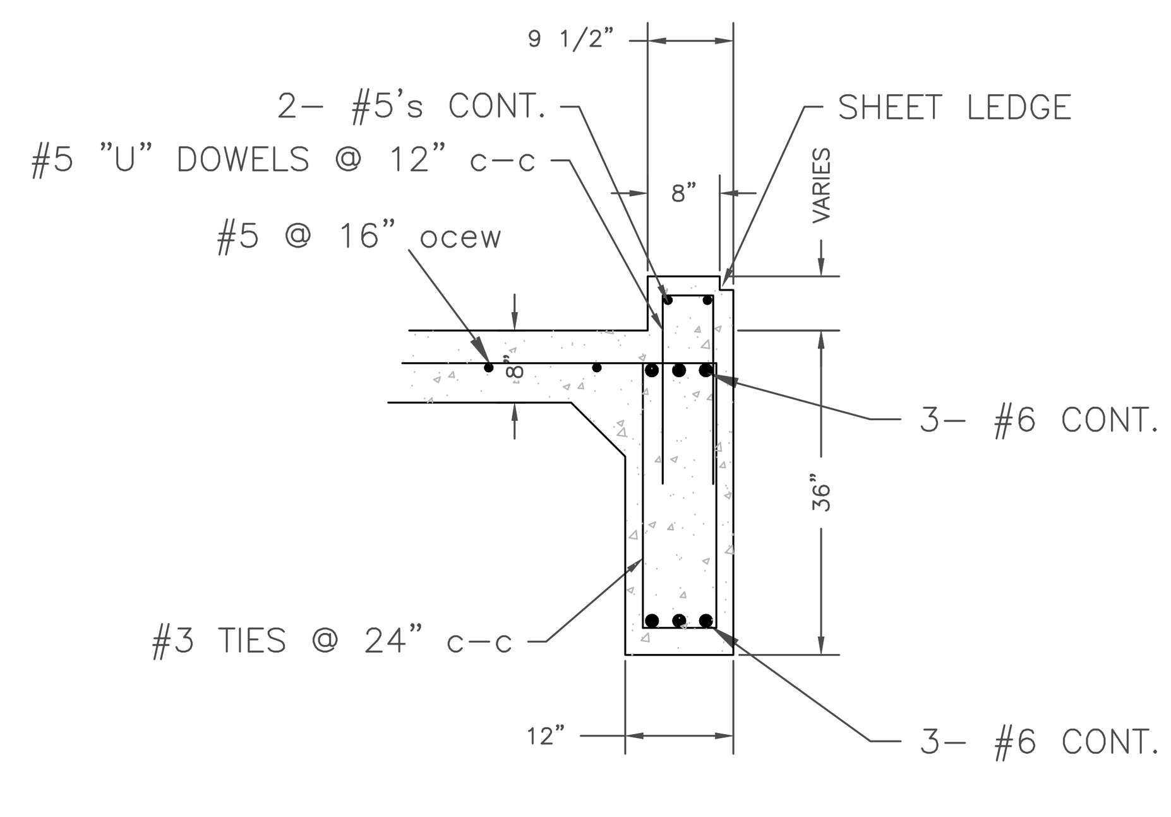
PROJECT NAME:
LSPI
LIQUID POWER FACILITY PRODUCTS, INC.
1331 INDEPENDENCE AVE.,
BRYAN TX. 77803
NON-AQUEOUS TOTE STORAGE BUILDING

DRAFTED BY:	DESIGNED BY:
SGH	PEM

SCALE: AS SHOWN

DRAWING TITLE:
PROPOSED FOUNDATION PLAN

DRAWING NO.:
S2



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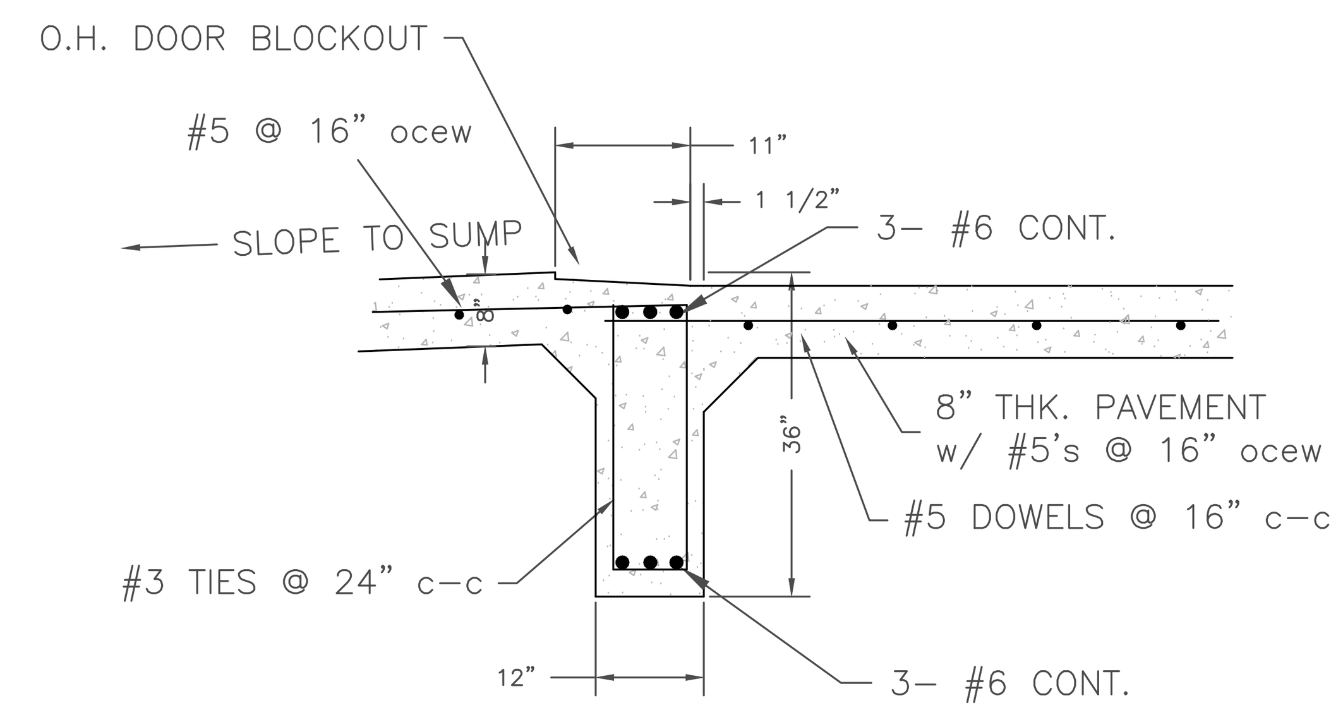
NTS



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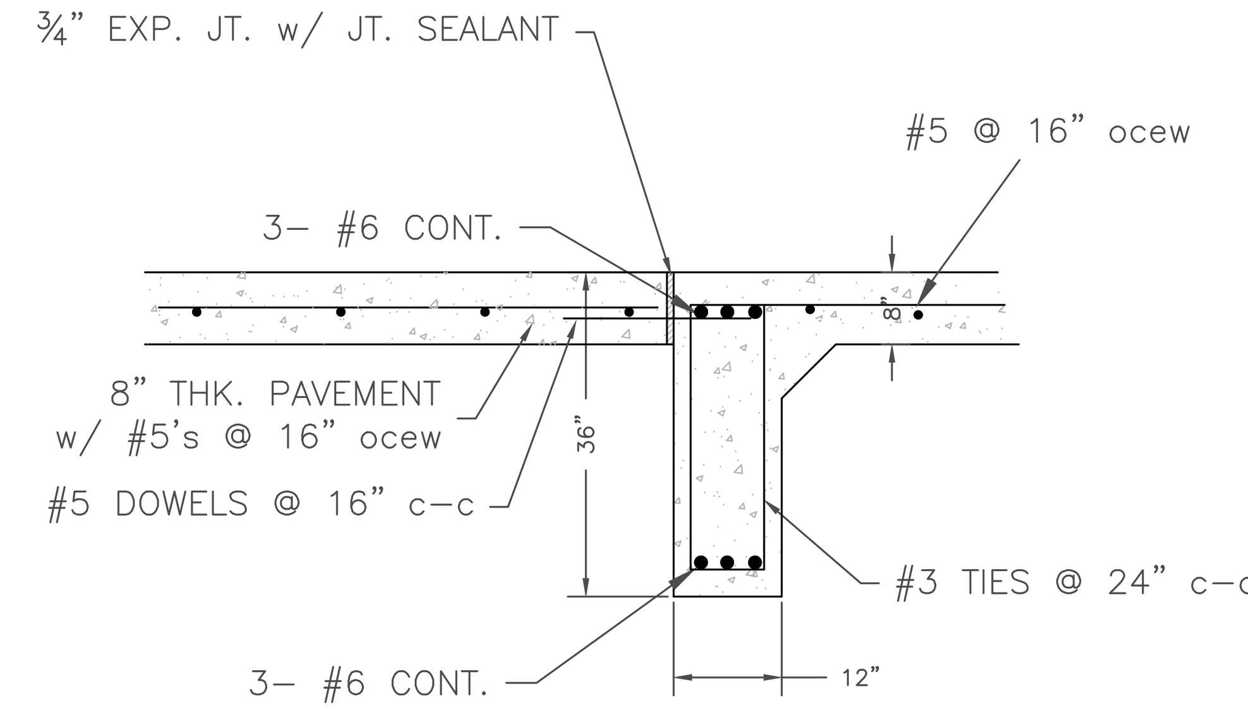

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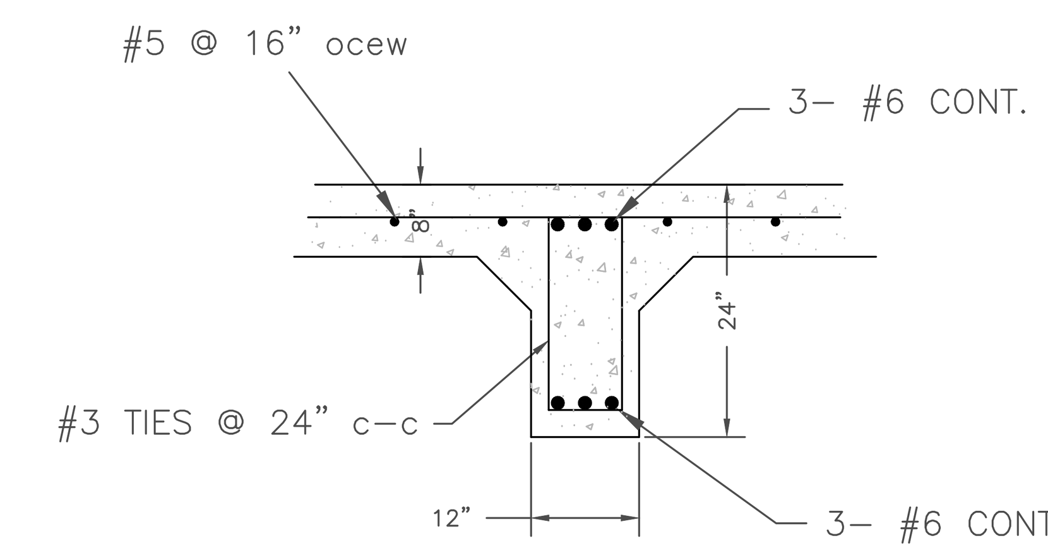

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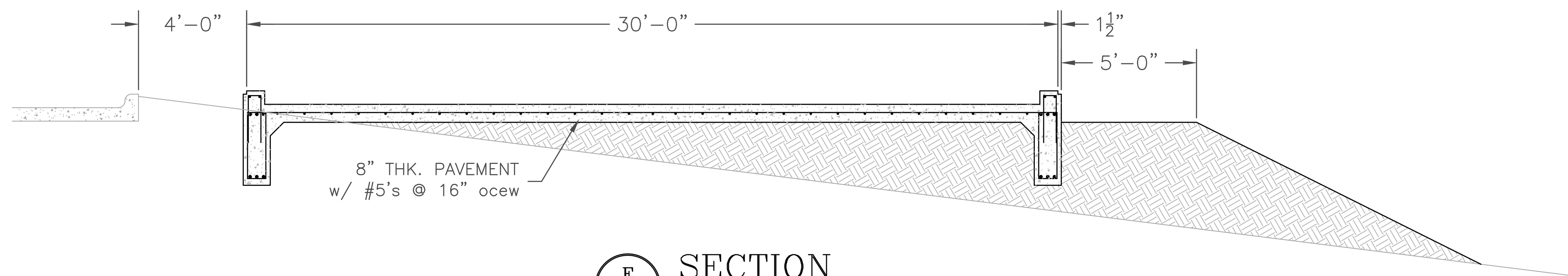

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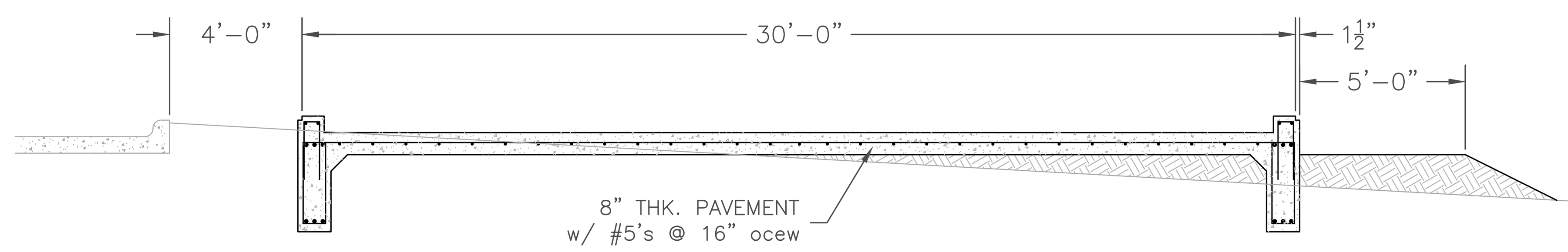

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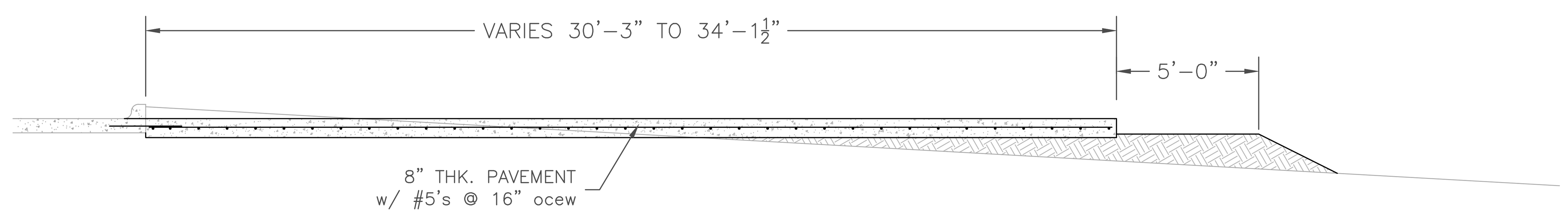

G S2 SECTION
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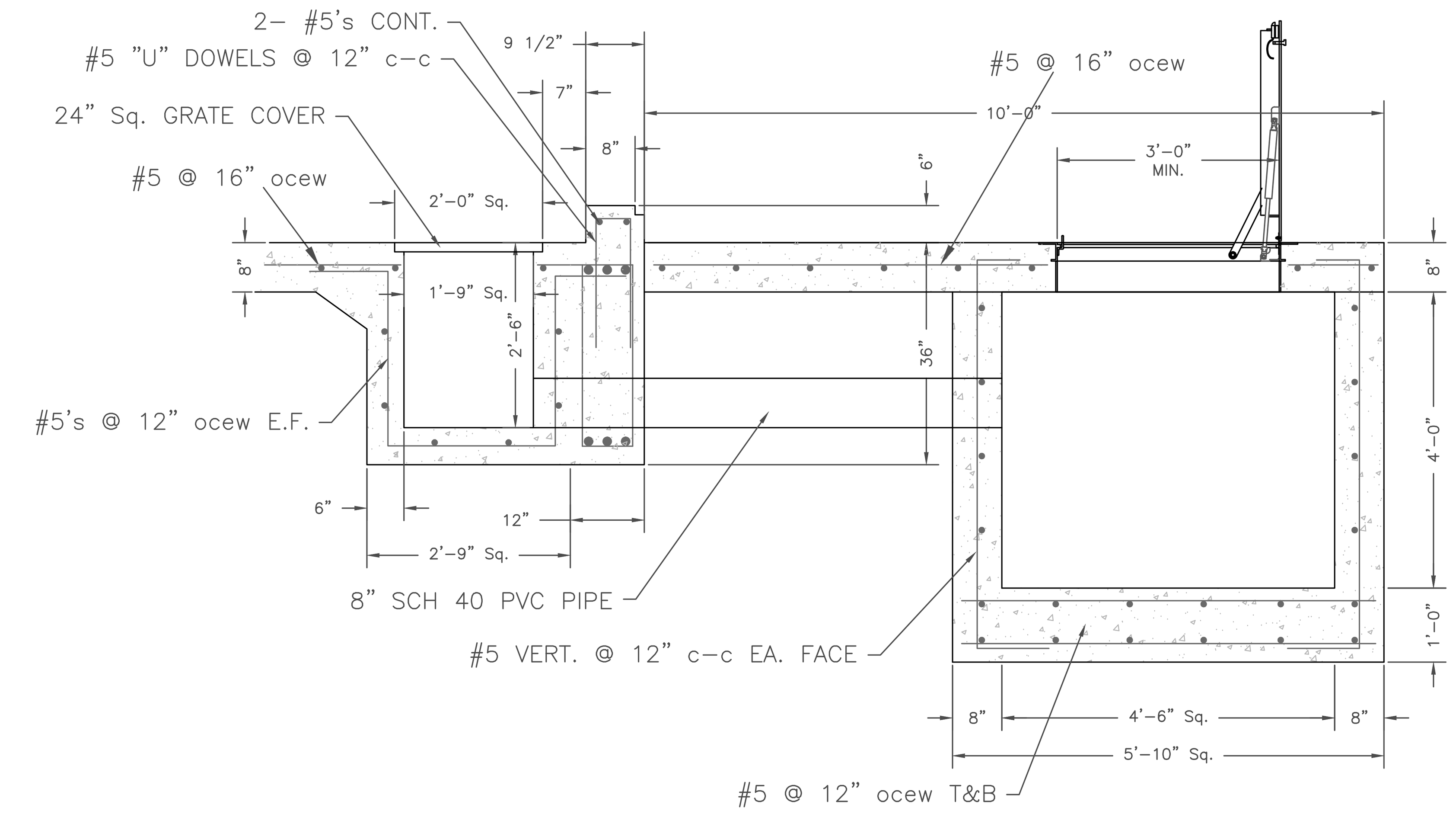
A S1 METAL BUILDING PLAN
SCALE: 1/4""=1'–0"



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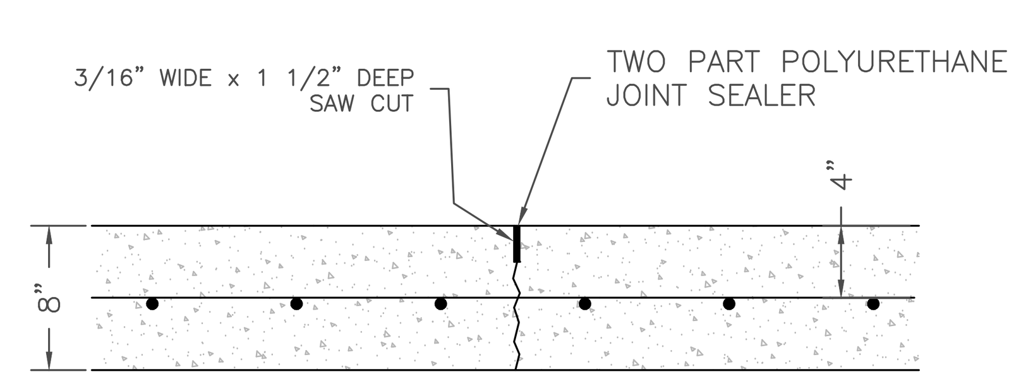
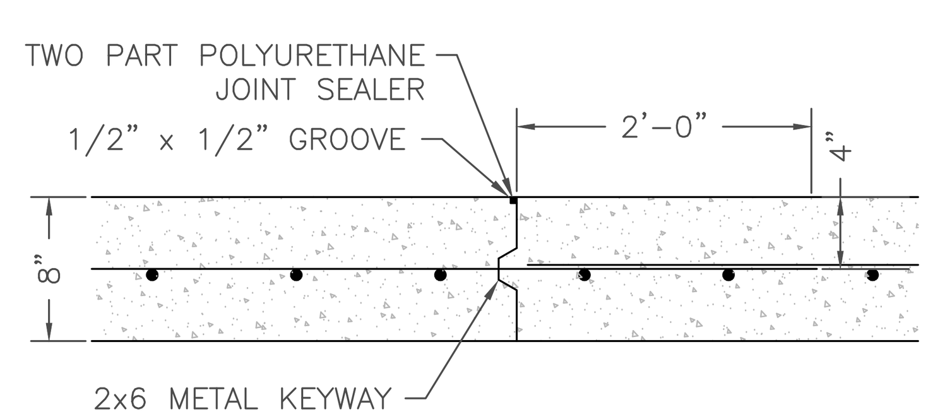


CONTROL JOINT DETAIL (CT. JT.)



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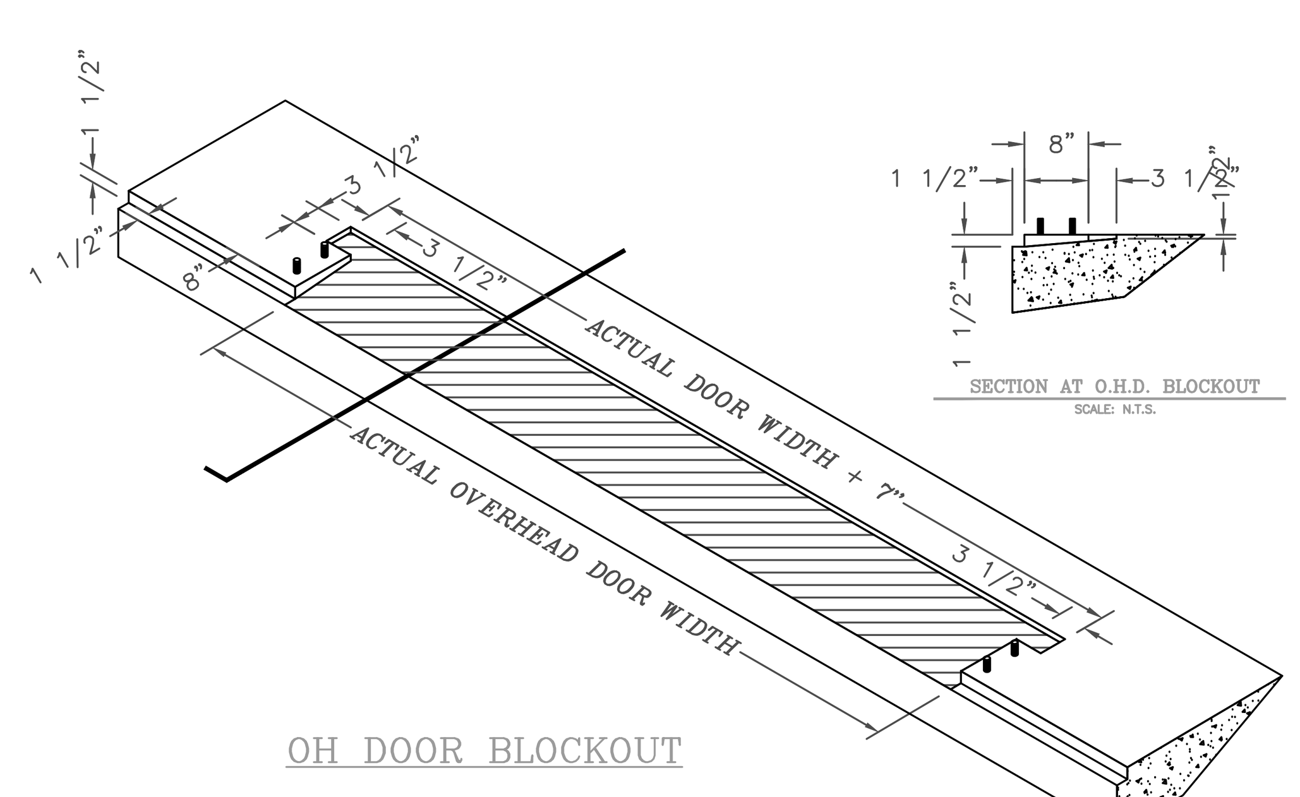
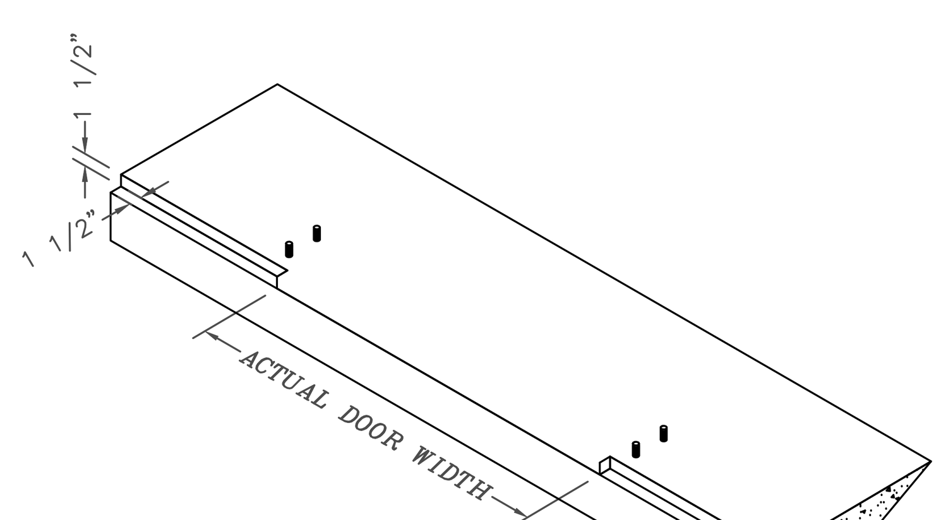
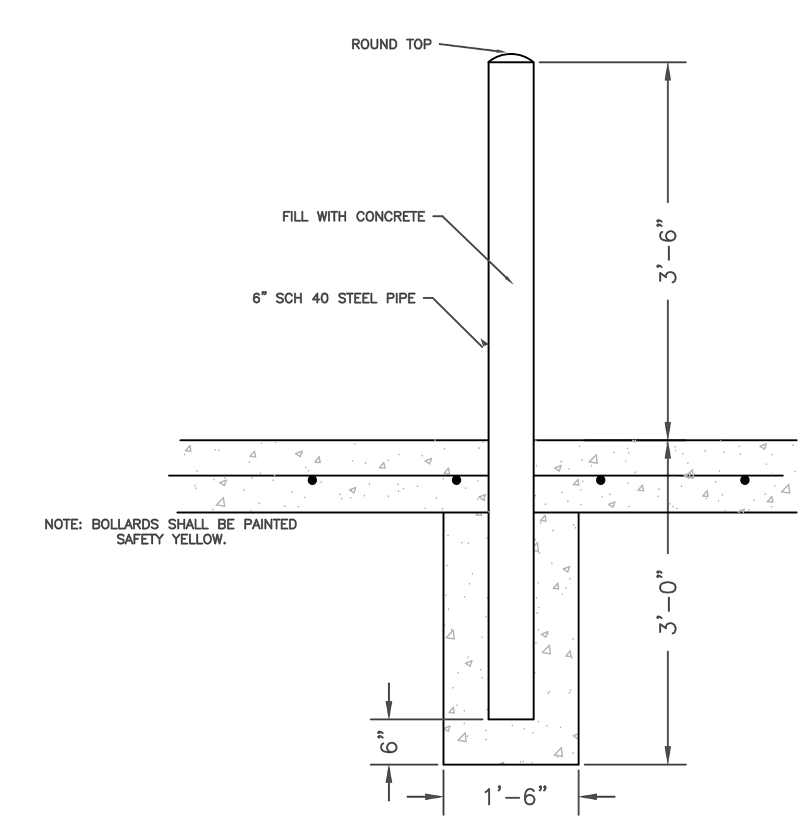


OH DOOR BLOCKOUT

SECTION AT O.H.D. BLOCKOUT



WALKDOOR DETAIL



BOLLARD DETAIL

NO.	REVISION DESCRIPTION	DATE	BY

STATE OF TEXAS
PAUL E. MALEK
82860
LICENSED PROFESSIONAL ENGINEER

DATE: 12/16/2025

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SCALE: AS SHOWN
DRAWING TITLE: PROPOSED BUILDING DETAILS
DRAWING NO.: S3